

variations between the boreholes or test pits. Subsurface conditions between boreholes or test pits may vary significantly from conditions encountered at the borehole or test pit locations.

GROUNDWATER

Where groundwater levels are measured in boreholes, there are several potential problems:

- Although groundwater may be present, in low permeability soils it may enter the hole slowly or perhaps not at all during the time it is left open;
- A localised perched water table may lead to an erroneous indication of the true water table;
- Water table levels will vary from time to time with seasons or recent weather changes and may not be the same at the time of construction; and
- The use of water or mud as a drilling fluid will mask any groundwater inflow. Water has to be blown out of the hole and drilling mud must be washed out of the hole or 'reverted' chemically if water observations are to be made.

More reliable measurements can be made by installing standpipes which are read after stabilising at intervals ranging from several days to perhaps weeks for low permeability soils. Piezometers, sealed in a particular stratum, may be advisable in low permeability soils or where there may be interference from perched water tables or surface water.

FILL

The presence of fill materials can often be determined only by the inclusion of foreign objects (e.g. bricks, concrete, plastic, slag/ash, steel etc) or by distinctly unusual colour, texture or fabric. Identification of the extent of fill materials will also depend on investigation methods and frequency. Where natural soils similar to those at the site are used for fill, it may be difficult with limited testing and sampling to reliably determine the extent of the fill.

The presence of fill materials is usually regarded with caution as the possible variation in density, strength and material type is much greater than with natural soil deposits. If the volume and quality of fill is of importance to a project, then frequent test pit excavations are preferable to boreholes

LABORATORY TESTING

Laboratory testing has not been undertaken to confirm the soil classifications and rocks strengths indicated on the environmental logs unless noted in the report.

SITE ANOMALIES

In the event that conditions encountered on site during construction appear to vary from those which were expected from the information contained in the report, EIS should be notified immediately.



GRAPHIC LOG SYMBOLS FOR SOIL AND ROCKS

		2001		DEFEC	
SOIL	FILL	ROCK	CONGLOMERATE	ZZZZ	TS AND INCLUSIONS CLAY SEAM
**************************************	TOPSOIL		SANDSTONE		SHEARED OR CRUSHED SEAM
	CLAY (CL, CH)	The state and st	SHALE	0000	BRECCIATED OR SHATTERED SEAM/ZONE
	SILT (ML, MH)		SILTSTONE, MUDSTONE, CLAYSTONE	4 4	IRONSTONE GRAVEL
	SAND (SP, SW)		LIMESTONE	e we we	ORGANIC MATERIAL
B 300	GRAVEL (GP, GW)		PHYLLITE, SCHIST	OTHER	R MATERIALS
	SANDY CLAY (CL, CH)		TUFF	74	CONCRETE
	SILTY CLAY (CL, CH)	三年	GRANITE, GABBRO		BITUMINOUS CONCRETE, COAL
X	CLAYEY SAND (SC)	* * * * * * * * * * * * * * *	DOLERITE, DIORITE		COLLUVIUM
	SILTY SAND (SM)		BASALT, ANDESITE		
3/9	GRAVELLY CLAY (CL, CH)		QUARTZITE		
\$ 6000 C	CLAYEY GRAVEL (GC)				
	SANDY SILT (ML)				
KATATA	PEAT AND ORGANIC SOILS				

	(Excluding par	rticles larger	ification Proceed than 75 µm and ated weights)	dures d basing fracti	ons on	Group Symbols	Typical Names	Information Required for Describing Soils		Laboratory Classification Criteria	
Corre-grained soils More than half of marerial is larger than 75 am slees size t particle visible to naked eye)	of coarse er than size	gravels e or no nes)	Wide range in grain size and substantial amounts of all intermediate particle sizes		GW	Well graded gravels, gravel- sand mixtures, little or no fines	Give typical name; indicate ap- proximate percentages of sand	sand from grain size ction smaller than 75 cclassified as follows: . SC	$C_{\text{U}} = \frac{D_{60}}{D_{10}} \text{Greater tha}$ $C_{\text{C}} = \frac{(D_{30})^2}{D_{10} \times D_{60}} \text{Bett}$	ween 1 and 3	
	wels alf of larger ieve siz	Clean gra (little or fines)			range of sizes sizes missing	GP	Poorly graded gravels, gravel- sand mixtures, little or no fines	and gravel; maximum size; angularity, surface condition, and hardness of the coarse grains; local or geologic name	from smaller ified as	Not meeting all gradation	requirements for GR
	Gravels More than half of fraction is large	with iable	Nonplastic fi	nes (for iden	ification pro-	GM	Silty gravels, poorly graded gravel-sand-silt mixtures	and other pertinent descriptive information; and symbols in parentheses	d sand d sand reclass V, SP V, SC ases red	"A" line, or PI less than 4	Above "A" lit with PI betwee 4 and 7 as
	More	Gravels with fines (appreciable amount of fines)	Plastic fines (t		on procedures,	GC	Clayey gravels, poorly graded gravel-sand-clay mixtures	For undisturbed soils add informa- tion on stratification, degree of compactness, cementation,	of gravel and of gravel and gravel and gravinod soils arr GW, GC, SW, GM, GC, SW dual symbo	Atterberg limits above "A" line, with PI greater than 7	borderline case requiring use dual symbols
	Sam Sam Sam Sam Sam Sam Sam Sam Sam Sam	n sands e or no nes)	Wide range is amounts of sizes	n grain sizes a: of all interme	nd substantial diate particle	SW	Well graded sands, gravelly sands, little or no fines	Example:		$C_{\rm U} = \frac{D_{60}}{D_{10}}$ Greater that $C_{\rm C} = \frac{(D_{30})^2}{D_{10} \times D_{40}}$ Betw	n 6 reen 1 and 3
		Clean	Predominanti with some	y one size or a intermediate	range of sizes sizes missing	SP	Poorly graded sands, gravelly sands, little or no fines	hard, angular gravel par- ticles 12 mm maximum size; rounded and subangular sand grains coarse to fine, about	given under fig ne percentages ng on percenta ve size) coarse ; than 5% to than 12%	Not meeting all gradation requirements for	
smallest p		Sands with fines (appreciable amount of fines)	Nonplastic fit	nes (for ident	ification pro-	SM	Silty sands, poorly graded sand- silt mixtures	15% non-plastic fines with low dry strength; well com- pacted and moist in place:	Determine curve Curve Depending mu sieve Less th More 1 5% to	"A" line or PI less than	Above "A" lin with PI betwee 4 and 7 as borderline case
t the sm	More 1 fractio	Sands fin (appre amou	Plastic fines (f	Plastic fines (for identification procedures, see CL below)		sc	Clayey sands, poorly graded sand-clay mixtures	alluvial sand; (SM)	fracti D	"A" line with PI greater than 7	
noq	Identification	Identification Procedures on Fraction Smaller than 380 µm Sieve Size					The second secon		the last		
iller s size is a			Dry Strength (crushing character- istics)	Dilatancy (reaction to shaking)	Toughness (consistency near plastic limit)				60 Comparis	ng soils at equal liquid limit	
oils rial is smaller e sire 5 µm sieve sia	and clays	Sits and clays liquid limit less than 50		Quick to	None	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands with slight plasticity	Give typical name; indicate degree and character of plasticity, amount and maximum size of coarse grains; colour in wet	40 Toughne	ss and dry strength increase	***
Finc-granned soils I half of material is I an 75 µm sieve size (The 75 µm si	Silts	E .	Medium to high	None to very slow	Medium	CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays	condition, odour if any, local or	Plasticity 00		OH
half n 75			Slight to medium	Slow	Slight	OL	Organic silts and organic silt- clays of low plasticity	For undisturbed soils add infor-	2 10 a		MH
ore than	200	Silts and clays liquid limit greater than 50		Slow to none	Slight to medium	мн	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts	mation on structure, stratifica- tion, consistency in undisturbed and remoulded states, moisture and drainage conditions	O MI	20 30 40 50 60 7	0 80 90 100
Mo	bid bid	8	High to very high	None	High	СН	Inorganic clays of high plas- ticity, fat clays	Example:		Liquid limit	
	Siltes		Medium to	None to very slow	Slight to medium	ОН	Organic clays of medium to high plasticity	Clayey silt, brown; slightly plastic; small percentage of	for labora	Plasticity chart atory classification of fin	e grained soils
н	lighly Organic S	Soils	Readily iden	tified by co	lour, odour,	PI	Peat and other highly organic soils	fine sand: numerous vertical root holes; firm and dry in place: locss; (ML)			

Note: 1 Soils possessing characteristics of two groups are designated by combinations of group symbols (eg. GW-GC, well graded gravel-sand mixture with clay fines) Soils with liquid limits of the order of 35 to 50 may be visually classified as being of medium plasticity.



LOG SYMBOLS

LOG COLUMN	SYMBOL	DEFINITION
Groundwater		Standing water level. Time delay following completion of drilling may be shown. Extent of borehole collapse shortly after drilling.
Record	—	Groundwater seepage into borehole or excavation noted during drilling or excavation.
Samples	ES U50 DB DS ASB ASS SAL	Soil sample taken over depth indicated, for environmental analysis. Undisturbed 50mm diameter tube sample taken over depth indicated. Bulk disturbed sample taken over depth indicated. Small disturbed bag sample taken over depth indicated. Soil sample taken over depth indicated, for asbestos screening. Soil sample taken over depth indicated, for acid sulfate soil analysis. Soil sample taken over depth indicated, for salinity analysis.
	N = 17 4, 7, 10	Standard Penetration Test (SPT) performed between depths indicated by lines. Individual show blows per 150mm penetration. 'R' as noted below.
Field Tests	N _c = 5 7 3 R	Solid Cone Penetration Test (SCPT) performed between depths indicated by lines. Individual figures show blows per 150mm penetration for 60 degree solid cone driven by SPT hammer 'R' refers to apparent hammer refusal within the corresponding 150mm depth increment.
	VNS = 25 PID = 100	Vane shear reading in kPa of Undrained Shear Strength. Photoionisation detector reading in ppm (Soil sample heads pace test).
Moisture (Cohesive Soils)	MC>PL MC≈PL MC <pl< td=""><td>Moisture content estimated to be greater than plastic limit. Moisture content estimated to be approximately equal to plastic limit. Moisture content estimated to be less than plastic limit.</td></pl<>	Moisture content estimated to be greater than plastic limit. Moisture content estimated to be approximately equal to plastic limit. Moisture content estimated to be less than plastic limit.
(Cohesionless)	D M W	DRY - Runs freely through fingers. MOIST - Does not run freely but no free water visible on soil surface. WET - Free water visible on soil surface.
Strength (Consistency) Cohesive Soils	VS S F St VSt H	VERY SOFT — Unconfined compressive strength less than 25kPa SOFT — Unconfined compressive strength 25-5 0kPa FIRM — Unconfined compressive strength 50-1 00kPa STIFF — Unconfined compressive strength 100- 200kPa VERY STIFF — Unconfined compressive strength 200- 400kPa HARD — Unconfined compressive strength greater than 400kPa Bracketed symbol indicates estimated consistency based on tactile examination or other tests.
Density Index/ Relative Density (Cohesionless Soils)	VL L MD D VD	Density Index (ID) Range (%) SPT ' N' Value Range (Blows/300mm) Very Loose < 15
Hand Penetrometer	300	Bracketed symbol indicates estimated density based on ease of drilling or other tests. Numbers indicate individual test results in kPa on representative undisturbed material unless noted otherwise
Readings Remarks	250 'V' bit 'TC' bit	Hardened steel 'V' shaped bit. Tungsten carbide wing bit.
	T ₆₀	Penetration of auger string in mm under static load of rig applied by drill head hydraulics without rotation of augers.



LOG SYMBOLS CONTINUED

ROCK STRENGTH

Rock strength is defined by the Point Load Strength Index (Is 50) and refers to the strength of the rock substance in the bedding. The test procedure is described by the International Journal of Rock Mechanics, Mining and Geomechanics Abstract Volume 22, No 2, 1985.

TERM	SYMBOL	Is (50) MPa	FIELD GUIDE					
Extremely Low:	EL	-0.03	Easily remoulded by hand to a material with soil properties.					
Very Low:	VL	0.00	May be crumbled in the hand. Sandstone is "sugary" and friable.					
Low:	L	0.1	A piece of core 150 mm long x 50mm dia. may be broken by hand and easily scored with a knife. Sharp edges of core may be friable and break during handling.					
Medium Strength:	М	0.3	A piece of core 150 mm long x 50mm dia. can be broken by hand with difficulty. Readily scored with knife.					
High:	Н	3	A piece of core 150 mm long x 50mm dia. core cannot be broken by hand, can be slightly scratched or scored with knife; rock rings under hammer.					
Very High:	VH	10	A piece of core 150 mm long \times 50mm dia. may be broken with hand-held pick after more than one blow. Cannot be scratched with pen knife; rockrings under hammer.					
Extremely High:	EH	.10	A piece of core 150 mm long x 50mm dia. is very difficult to break with h and-held hammer . Rings when struck with a hammer.					

ROCK STRENGTH

ABBREVIATION	DESCRIPTION	NOTES
Be CS	Bedding Plane Parting Clay Seam	Defect orientations measured relative to the normal to (i.e. relative to horizontal for vertical holes)
J	Joint	
P	Planar	
Un	Undulating	
	Smooth	
S R	Rough	
IS	Iron stained	
xws	Extremely Weathered Seam	
Cr	Crushed Seam	
60t	Thickness of defect in millimetres	



Appendix B: Laboratory Reports and COC Documents

TO: ENVIROLAB SERVICES PTY LTD 12 ASHLEY STREET CHATSWOOD NSW 2067				EIS Job Number:		E29353KM	FROM: ENVIRONMENTAL INVESTIGATION SERVICES								EIS		
P: (02) 99106200 F: (02) 99106201		Date Res	STANDARD	And a Maria Control of the Control o							RK, NSW 2113						
Attention: Aileen				Page:		1 of 1	end-				Atter	ntion:			Rob I	Mulle	r
Location:	Dolls F	Point								Samp			ed in l		on Ice		
Sampler:	Arthur	Billingham	T	T			Tests Required										
Date Sampled	Lab Ref:	Sample Number	Depth (m)	Sample Container	PID	Sample Description	SPOCAS	Combo 3a	Combo 6	Combo 6a	8 Metals	PAHs	TRH/BTEX	втех	Asbestos		
4/05/2016	(BH1	0.0-0.1	G, A	0	Fill				X							
3/05/2016	1	BH2	0.0-0.1	G, A	0	Fill				x							
3/05/2016	3	BH2	0.5-0.7	G, A	0	Sand				x							
5/05/2016	4	внз	0.4-0.5	G, A	0	Fill				x							
2/05/2016	2	BH4	0.0-0.1	G, A	0	Fill				X							
4/05/2016	6	BH1	3.0-3.15	Р	-	Sand	X										
3/05/2016	7	BH2	5.7-6.15	Р	-	Sand	x										
5/05/2016	8	внз	7.2-7.65	Р	-	Silty sand	X										
2/05/2016	9	BH4	7.2-7.65	Р	-	Sand	x										
								4		eń	VIROL	98	En	virola:	Serv	ices	
												1	Chats Ph	wood (02)	Ashle NSW 9910	2007	
										Dat	te Rec	eive	46	5/			
										Coo	ding. I	ol/An	bient paek Proke				
													211386	en/No	ne		
															- 2		
			limits required				G - 2 A - Z	50mg			Bag						
Relinquished	By:	441/	17	Date:	15/1	б	Time	:			Rece	eived I	Ву:			Date	



Envirolab Services Pty Ltd
ABN 37 112 535 645
12 Ashley St Chatswood NSW 2067
ph 02 9910 6200 fax 02 9910 6201
enquiries@envirolabservices.com.au
www.envirolabservices.com.au

SAMPLE RECEIPT ADVICE

Client Details	的复数形式 医克里特氏 医二甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基
Client	Environmental Investigation Services
Attention	Rob Muller

Sample Login Details		
Your Reference	E29353KM, Dolls Point	
Envirolab Reference	146263	
Date Sample Received	09/05/2016	
Date Instructions Received	09/05/2016	
Date Results Expected to be Reported	16/05/2016	

Sample Condition	
Samples received in appropriate condition for analysis	YES
No. of Samples Provided	9 Soils
Turnaround Time Requested	Standard
Temperature on receipt (°C)	3.7
Cooling Method	Ice Pack
Sampling Date Provided	YES

omments
amples will be held for 1 month for water samples and 2 months for soil samples from date eccipt of samples

Please direct any queries to:

Aileen Hie	Jacinta Hurst
Phone: 02 9910 6200	Phone: 02 9910 6200
Fax: 02 9910 6201	Fax: 02 9910 6201
Email: ahie@envirolabservices.com.au	Email: jhurst@envirolabservices.com.au

Sample and Testing Details on following page



Envirolab Services Pty Ltd
ABN 37 112 535 645
12 Ashley St Chalswood NSW 2067
ph 02 9910 6200 fax 02 9910 6201
enquiries@envirolabservices.com.au
www.envirolabservices.com.au

Sample Id	vTRH(C6- C10)/BTEXN in Soil	svTRH (C10-C40) in Soil	PAHs in Soil	Organochlorine Pesticides in soil	Organophosphorus Pesticides	PCBs in Soil	Acid Extractable metals in soil	Asbestos ID - soils	sPOCAS
BH1-0.0-0.1	1	1	1	1	1	1	1	1	
BH2-0.0-0.1	1	1	1	/	1	1	1	/	
BH2-0.5-0.7	1	1	1	1	1	1	1	1	
BH3-0.4-0.5	1	1	1	1	1	1	1	1	
BH4-0.0-0.1	1	1	1	1	1	1	1	1	
BH1-3.0-3.15									1
BH2-5.7-6.15									1
BH3-7.2-7.65									1
BH4-7.2-7.65									1







email: sydney@envirolab.com.au envirolab.com.au

Envirolab Services Pty Ltd - Sydney | ABN 37 112 535 645

CERTIFICATE OF ANALYSIS

146263

Client:

Environmental Investigation Services
PO Box 976
North Ryde BC
NSW 1670

Attention: Rob Muller

Sample log in details:

Your Reference: E29353KM, Dolls Point

No. of samples: 9 Soils

Date samples received / completed instructions received 9/5/2016 / 9/5/2016

Analysis Details:

Please refer to the following pages for results, methodology summary and quality control data.

Samples were analysed as received from the client. Results relate specifically to the samples as received. Results are reported on a dry weight basis for solids and on an as received basis for other matrices.

Please refer to the last page of this report for any comments relating to the results.

Report Details:

Date results requested by: / Issue Date:

16/05/16

16/05/16

Date of Preliminary Report:

Not Issued

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Accredited for compliance with ISO/IEC 17025.

Tests not covered by NATA are denoted with *.

Results Approved By:

Jacinta/Hurst Laboratory Manager

Envirolab Reference:

146263

Revision No:

R 00

